# Beveridge Williams development & environment consultants



# 379 Lang Lang-Poowong Road, Nyora

**Stormwater Management Strategy** 

Wallis Watson Nyora Pty Ltd

October 2018

#### DOCUMENT CONTROL DATA



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www.beveridgewilliams.com.au		379 Lang Lang-Poowong Road, Nyora

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# **CONTENTS**

1	INTR	ODUCTION3
	1.1	SITE OVERVIEW3
	1.2	SITE CONTEXT4
2	EXIST	ring conditions5
	2.1	TOPOGRAPHY5
	2.2	SURFACE WATER AND DRAINAGE6
3	DESI	GN INTENT7
	3.1	PROPOSED DEVELOPMENT7
	3.2	PROPOSED STORMWATER MANAGEMENT STRATEGY7
4	STOR	MWATER QUANTITY MANAGEMENT9
	4.1	HYDROLOGY9
		4.1.1 MODEL CALIBRATION/VALIDATION
	4.2	100-YEAR ARI FLOOD RESULTS11
	4.3	SUB-SURFACE DRAINAGE
	4.4	OVERLAND FLOW12
	4.5	SIZING OF THE DETENTION BASIN
5	STOR	MWATER QUALITY MANAGEMENT14
6	STAG	ing Plan17
	6.1 POOV	STAGING OF STORMWATER MANAGEMENT ASSETS FOR 379 LANG LANG – VONG ROAD17
7	CON	CLUSION18

# **APPENDICES**

APPENDIX A. SITE TOPOGRAPHY PLA
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APPENDIX B. INDICATIVE SUBDIVISION PLAN

APPENDIX C. PRE- AND POST- DEVELOPMENT PLANS

APPENDIX D. RORB CALCULATION RESULTS

APPENDIX E. WSUD ASSETS AND DETENTION BASIN CONCEPT PLAN

# Glossary of terms

Alphabetical list of terms and abbreviations used in report

AHD Australian Height Datum

A common national surface level datum approximately corresponding to mean sea

level.

ARI Average Recurrence Interval - The average, or expected, value of the periods between

exceedances of a given rainfall total accumulated over a given duration.

Authorities Organisations responsible for supply and management of sewer, water, gas, electricity

and telecommunications, roads and transport

BPEMG Best Practice Environmental Management Guidelines

BW Beveridge Williams
MW Melbourne Water

Client Wallis Watson Nyora Pty Ltd
Council South Gippsland Shire Council
IDM Infrastructure Design Manual

Q<sub>5</sub> Stormwater flow generated from 5 year ARI storm event.
 Q<sub>100</sub> Stormwater flow generated from 100 year ARI storm event.

 $Q_{\text{gap}}$  Flow difference between  $Q_5$  and  $Q_{100}$  storm event.

SEPP State Environment Protection Policy
SWMS Stormwater Management Strategy

WSUD Water Sensitive Urban Design

#### 1 INTRODUCTION

Beveridge Williams has been commissioned by Wallis Watson Nyora Pty Ltd (the Client) to prepare a Stormwater Management Strategy (SWMS) for a proposed residential development, located at 379 Lang Lang-Poowong Road, Nyora. Due to the nature of the local topography, however, a combined drainage strategy inclusive of the neighbouring lots of 30 Glovers Rod and 951 Yannathan Road is required to satisfy the Development Planning Overlay (DPO) — shown on planning scheme maps as DPO10. These three properties are referred to as "the site" within this document. The site area is approximately 103 ha and it is proposed to develop this land into a mixture of low and conventional density residential lots.

This SWMS report is intended to provide a conceptual drainage strategy for the development plan (APPENDIX A). The strategy aims to retain post development stormwater runoff to pre-development levels and to meet stormwater quality Best Practice Environmental Management Guidelines (BPEMG) to the satisfaction of Melbourne Water, South Gippsland Shire Council and other relevant authorities.

#### 1.1 Site Overview

The site is currently vacant land and is bounded by Lang Lang-Poowong Road to the south, Glovers Road to the east and north and Yannathan Road to the west. The site is currently accessible from each road.

There is a large existing dam located on the north-western part of 379 Lang Lang-Poowong Road and there are existing watercourses and other dams also located on this property (Refer to Site Aerial Map in Figure 1 below).



Figure 1: Site Aerial Map (Source: Google Earth)

#### 1.2 Site Context

The site lies within the South Gippsland Planning Scheme area. The site is generally separated into two zones, General Residential Zone(GRZ1) and Farming(FZ). The majority of 379 Lang Lang – Poowong Road is zoned General Residential Zone (GRZ1) and the remaining components of the site's properties are currently zoned Farming (FZ) (See Figure 2 below).



Figure 2: Zoning Map Plan (Source: South Gippsland Planning Schemes - Online)

Contextually, this SWMS is primarily focused on 379 Lang Lang – Poowong Road, however, this combined strategy has been developed for a holistic approach to the total development of the entire site.

It is recognised that at the time of this strategy's development, large portions of the site are zoned as FZ, and are, therefore, not available for development. The strategic deployment of assets within this document reflects the current situation and provides for the future development of the eastern and western portions of the subject site outlined above.

#### **2 EXISTING CONDITIONS**

## 2.1 Topography

Generally speaking, the site flows from south to north, from an approximate maximum elevation of 127m AHD to an approximate 104m AHD.

The site's northwestern lobe is transected by a low-lying ridge which directs a minor portion of runoff from 30 Glovers Road and the upstream external catchment towards 379 Lang Lang-Poowong Road, generally sloping in a southeasterly direction towards the existing waterway. A small knoll is centrally located within 379 Lang Lang-Poowong Road, locally interrupting the site's general slope from south to north. The existing waterway on 379 Lang Lang Poowong Road flows from south to north towards Glovers Road and ultimately deposits flows into farm dams further downstream.

A plan showing the site topography is shown in Figure 3 below.

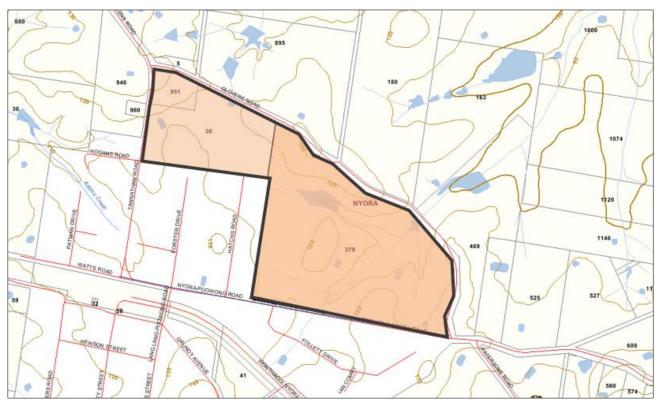


Figure 3: Site Topography Plan (Not to Scale) (Source: DEWLP-land.vic.gov.au)

# 2.2 Surface Water and Drainage

Overland flows from the high points on the south run north, towards the existing waterway and further northern direction. The north-west part of the site is primarily flat and flows towards both north and east as governed by the low-lying ridge (Refer to Figure 4 below).

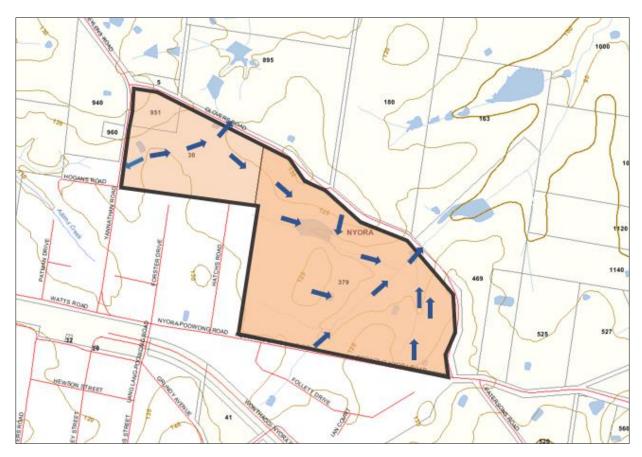


Figure 4: Site Analysis Plan (Not to Scale) (Source: DEWLP-land.vic.gov.au)

#### 3 DESIGN INTENT

## 3.1 Proposed Development

The proposed residential subdivision is shown in the Indicative Development Plan in Figure 5 below. The site area is 103 ha and it is proposed to primarily consist of standard and low density residential lots. The low density lots will be located on the north-west corner, south east corner and along the western boundary, while the conventional density lots will occupy the remaining area of the site.

This preliminary development layout also includes an active open space, a passive open space reserve and four drainage reserve areas.

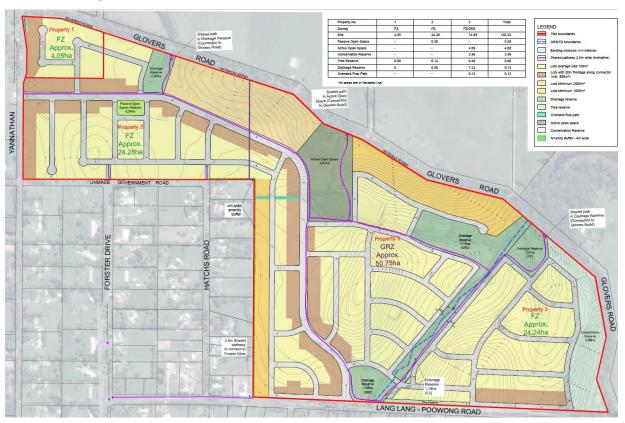


Figure 5: Indicative Overall Development Plan for Lang Lang-Poowong Road, Nyora (Not to Scale)

#### 3.2 Proposed Stormwater Management Strategy

Discussions with Council have indicated that the proposed drainage system is to be designed for the holistic treatment of the catchment, with the nominated drainage reserves serving to treat and retard stormwater they have received from upstream portions of the site and catchment. This requires that the sizing of assets consider the total flows and pollutant loads to be received, even if the origin of those flows is external to the site or the property upon which the asset is located.

This SWMS has been proposed to follow the existing natural features of the pre-developed site which include:

- Controlling the rate of the 1 in 100-year ARI stormwater runoff for the post-development peak flows to pre-development levels.
- Providing stormwater treatment to meet the best practice guidelines.

Ownership of the properties comprising the site and outlined in Figure 5 is not consistent. The design and construction of drainage assets proposed within this strategy will therefore be a shared responsibility between the landowners of the properties that each asset services.

#### **Stormwater Quantity Management**

For stormwater quantity management, it is proposed to provide four stormwater detention basins across the development of the overall site. The detention basins will serve to retard stormwater runoff for the 100-year ARI storm event on the development back to pre-development peak flows.

The basin locations are as follows:

#### Within 379 Lang Lang-Poowong Road:

Three retarding basins

#### • Within 30 Glovers Road:

One retarding basin

#### **Stormwater Quality Management**

The sedimentation basins, wetlands and bioretention basin will provide the required stormwater treatment to achieve the best practice standard. For stormwater quality management, it is proposed to provide two sedimentation basins, two wetlands and a bioretention basin within each of the stormwater detention basins on the subject site. In addition, a swale is also proposed from the southern to north eastern boundary which will provide the required stormwater treatment to achieve the best practice standard.

The location of the proposed basins/wetlands is as follows:

#### • Within 379 Lang Lang-Poowong Road:

- Two sedimentation basins
- Two wetlands
- o One swale

#### Within 30 Glovers Road:

One bioretention basin

Details of both stormwater quantity and quality management are discussed in detail in Sections 4 & 5.

#### **Legal Point of Discharge and Waterway**

The Legal Point of Discharge (LPD) for 379 Lang Lang Poowong Road will be to the waterway across Glovers Road on the external north east of the site. LPD for 30 Glovers Road and 951 Yannathan Road will be to the north of Glovers Road, however, as these properties are presently zone as FZ, this may be subject to change.

It is proposed to maintain the existing condition of the waterway on the subject site, with minor landscape modification surrounding the bank.

## 4 STORMWATER QUANTITY MANAGEMENT

It is part of Melbourne Water and Council requirements that the 1 in 100-year ARI post development peak flow be detained back to the pre-development levels. The following sections provide further details of the stormwater quantity management for the proposed development site.

#### 4.1 Hydrology

The hydrological analysis of the 1 in 100-year ARI flows for the proposed development site was undertaken using the RORB runoff routing program model to determine the design flows of the predeveloped and developed scenarios. The following sub-sections provide details on the model calibration, catchment plans and the post-development flows results.

#### 4.1.1 Model Calibration/Validation

The pre-development and post development catchment plans were developed to include the entire catchment, as shown in Figure 6 and Figure 7, respectively (also in APPENDIX B). The post development catchment plan shows a series of sub-areas that indicate where stormwater discharge is channelled towards the existing dam to the east of the site.

The adopted RORB model parameters were based on calibration against the peak flows obtained using the Rational Method, whereby the  $k_c$  parameter for the pre-development scenario was determined by calibrating the RORB result to match the result obtained from the Rational Method. The  $k_c$  parameter for the post development scenario was determined by using a  $k_c$  to  $d_{av}$  ratio from the calibrated pre-development parameter. Details of these parameters are provided in Table 1.

**Table 1: RORB Input Parameters** 

Parameter	Adopted Parameters for the Pre Development Scenario	Adopted Parameters for the Post Development Scenario	
<b>k</b> <sub>c</sub>	1.30	1.15	
d <sub>av</sub>	1.14 km	1.01 km	
<b>m</b> 0.8		0.8	
IL 10.00		10.00	
CROp	0.6 (100yr)	0.6 (100yr)	

Further, the RORB model sub-areas and fraction impervious used for the proposed detention basin are provided in Table 2 below.

**Table 2: RORB Sub-Catchment Areas and Fraction Impervious** 

RORB Catchment Area (Overall sites – inclusive of 3 sites)	103 ha
RORB External Catchment Area to the south	93.2 ha
Fraction Impervious (Residential-Conventional Density)	0.6
Fraction Impervious (Residential-Low Density)	0.2
Fraction Impervious (Reserve)	0.1

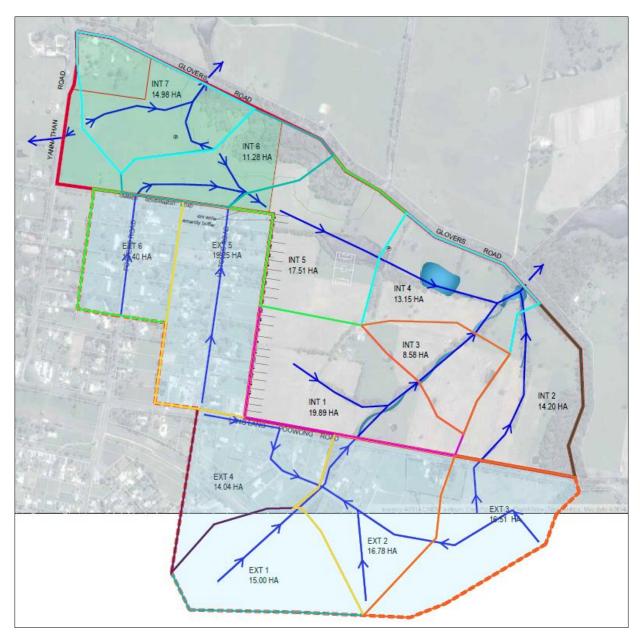


Figure 6: Pre-Development Catchment Plan



Figure 7: Post Development Catchment Plan

#### 4.2 100-year ARI Flood Results

The results of 1 in 100-year ARI peak pre-development and post development flows from the RORB modelling are shown in Table 3 below. Details of the RORB modelling results are shown in APPENDIX C.

 Table 3: RORB Model Results for the 100-year ARI Post Development (Critical Durations in parentheses)

100 year A	Storage Volume Required	
Peak Pre-Development Flow (m³/s)	Peak Post Development Outflow (m³/s) (with Detention)	for Detention (m <sup>3</sup> )
8.17 (2hr)	8.01 (2hr)	25,200 m³

The above peak flows results indicate that the 1 in 100-year ARI post development peak flows can be detained to the pre-development level by providing a detention storage of 25,200 m<sup>3</sup>.

## 4.3 Sub-surface Drainage

As previously mentioned, the LPDs for the site will be to the existing outfall/waterway(s) along the northern boundary based on the property from which flows are being disposed.



The subsurface drainage network for the development will convey all pipe flows to these locations, via the proposed water quality treatment facilities, which are the sedimentation basin, wetland, Bioretention basin and Swale. The pipe network will be adequately sized to convey the 1 in 5-year ARI flows throughout the proposed development's drainage network.

In addition, there is an external catchment (existing residential area lots fronting Lang Lang-Poowong Road) to the south of the site that will be directed to the proposed sub-surface drainage network.

#### 4.4 Overland Flow

Overland flows will be conveyed across the site to the nominated retarding basin assets for the specific portion of the catchment in which the flows originate. Due to the slope constraints, overland flows from lots on 951 Yannathan Road and on the western portion of 30 Glovers Road will be directed to the asset located on 30 Glovers Road (Refer to the Overland Flow Path Plan in Figure 9).

Retarding basin assets will have a controlled outlet whose design will be completed as part of the detailed design.

The internal roads for the development and the finished surface levels of the lots will be designed to ensure that the 1 in 100-year ARI overland flows through the site are within the safe hydraulic capacity of road floodway. A PC Convey assessment of the road reserve shows that the 100-year ARI gap flow can be contained within floodway safety criteria. A typical cross section is shown in Figure 8 and the calculation result is included in APPENDIX D.

The section location was chosen as it is the largest peak gap flow would be expected to provide the most conservative results (See Figure 9 for location of the assessment). The flood conveyance of the road reserves is within the flood safety criteria.

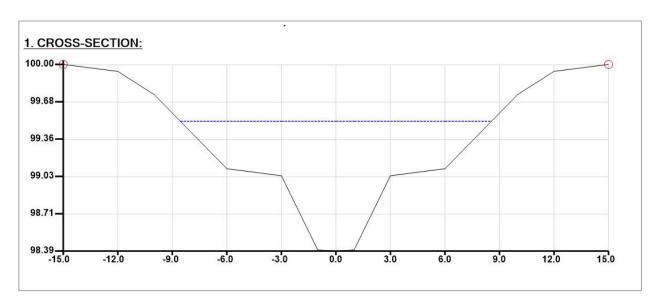
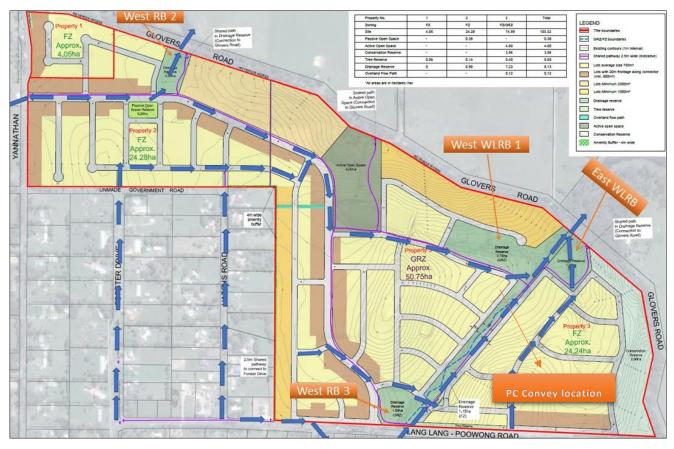


Figure 8: Typical Road Cross-Section showing the 100 year ARI Flood Level



**Figure 9: Indicative Overland Flow Path** 

# 4.5 Sizing of the Detention Basin

Details of the proposed retarding basins are provided in Table 4 below and a plan of the indicative locations is provided in the previous Figure 9 (Also APPENDIX D).

The retarding basin assets will over-detain the 100-year ARI flow to allow the northern low density residential development runoff to be free draining. The total flow from the site is expected to be less than the pre-developed flow for design storms up to the 100-year ARI event.

**Table 4: Details of Detention Basins** 

	Outlet Cont	Water	Storage	
Retarding Basin Pipe Outlet		Crest Elevation/Width for Weir (100- year weir crest)		Level at 100-year ARI
West RB 2	3 x 600 mm dia pipe	1 m /10 m	1.03 m	5,160 m <sup>3</sup>
West RB 3	525 mm dia pipe	1 m/ 10 m	0.93 m	3,930 m³
West WLRB 1	4 x 600 mm dia pipe	1 m / 10 m	0.99 m	9,900 m³
East WLRB	600 mm dia pipe	1 m / 10 m	1.01 m	6,210 m³

#### 5 STORMWATER QUALITY MANAGEMENT

Melbourne Water require all developments to provide a minimum best-practice water quality treatment via 'at source' methods. The current standards (BPEMG Targets) are 70/80/45/45 percent removal of Gross Pollutants/Total Suspended Solids/Total Phosphorous/Total Nitrogen (GP/TSS/TP/TN), respectively.

Stormwater quality modelling was conducted using MUSIC (Model for Urban Stormwater Improvement Conceptualisation) for the proposed development site. The layout of the MUSIC Model is shown in Figure 10 and results of the MUSIC model are shown in Table 5. The proposed treatment for catchments contributing flows to 379 Lang Lang-Poowong Road will be two wetlands (each with a sedimentation basin). One of the wetlands (WLRB West) will be located adjacent to the current location of the existing dam while the other (WLRB East) will be located on the Eastern of the site.

The proposed treatment train for the catchments contributing flows to 30 Glovers Road, including the external catchments, will be a sedimentation and bioretention basin.

Stormwater quality treatment for the southern part of the site will be carried out via the proposed swale.

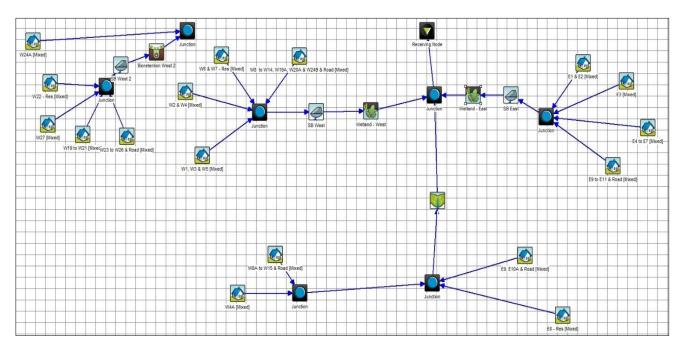


Figure 10: MUSIC Model Layout

Table 5: MUSIC Model Results-Compared with BPEMG Target

Parameter	% Removal at Receiving Outlet	BPEMG Target % Removal
Gross Pollutants (Kg/yr)	100.0	70
Total Suspended Solids (TSS) (Kg/yr)	84.3	80
Total Phosphorus (TP) (Kg/yr)	68.4	45
Total Nitrogen (TN) (Kg/yr)	45.2	45

As shown in Table 5, the proposed Bioretention Basin, sedimentation basins, wetlands and swales can meet the best practice BPEMG targets for the site.

Details of stormwater treatments assets are shown in Table 6 and Table 7.

**Table 6: Details of Sedimentation Basins and Wetlands** 

Ass	set	Catchment Area	Surface Area	Permanent Pool Volume
	SB West 2	25.54 ha	500 m²	N/A
Sedimentation Basins	SB West	39.56 ha	800 m²	N/A
2455	SB East	19.32 ha	360 m²	N/A
Watlands	WLRB 1	39.56 ha	8,000 m²	1,750 m³
Wetlands	East WLRB	19.32 ha	3,500 m²	4,000 m³

**Table 7: Dimension Details of Swale** 

Catchment Area	14.99 ha (proposed development)
Swale Top Width	8.0 m
Swale Bottom Width	2.0 m
Swale Length	430 m
Swale Depth	0.5 m
Swale Side Batter	1 in 6

The sedimentation basins are to be located in the base of the detention basins and the wetlands will be designed in accordance with Melbourne Water's Wetland Design Guidelines (as per the Wetland Design Manual). The design of the stormwater quality assets will be undertaken during the functional and detailed design phases of the project and submitted to Council and Melbourne Water at this time for comment and approval. The configuration of the outlet control structure will also be completed during the detailed design.

The concept design layout of the sedimentation basins and wetlands are shown in Figure 11. The concept design plans, including layout and cross sections of the stormwater treatment assets are included in APPENDIX D.



Figure 11: Concept Design Plan of the WSUD Assets (Not to Scale)

#### 6 STAGING PLAN

As previously discussed, the majority of the site is currently zoned as FZ, precluding it from residential development at this time. Consequently, the preliminary development of the site is currently proposed to occur over 13 stages for the portion of 379 Lang Lang - Poowong Road zoned for residential development (GRZ1). The Indicative Subdivision and Staging Plan for these 13 stages is provided in APPENDIX E.

It is anticipated that the eastern component of 379 Lang Lang - Poowong Road will be rezoned for residential development in the future, and this region is therefore considered to be a long term residential area subject to this rezoning. Subsequent amendments to the zoning for the remainder of the site are anticipated, however, the staging of this potential development is not currently available.

#### 6.1 Staging of Stormwater Management Assets for 379 Lang Lang – Poowong Road

The stormwater assets proposed in Sections 4 and 5 of this document have been conceptually designed in order to enable their construction in a staged manner complementing the development of the site.

To this end, West RB3, West WLRB 1 and the swale are proposed to be constructed to service Stages 1 through 13 of the staging plan, with East WLRB proposed to be constructed in line with the rezoning of the eastern component of 379 Lang Lang – Poowong Road. All remaining assets are to be constructed by the owners of the remaining two properties to service their portions of the site's development.

This approach will eliminate the need for interim assets.

#### 7 CONCLUSION

This report has identified an overall drainage management strategy for the proposed development site located across the following properties:

- 379 Lang Lang-Poowong Road, Nyora;
- 30 Glovers Road, Nyora; and
- 951 Yannathan Road, Nyora.

The strategy provides methodologies for the management of stormwater on the site, which are as follows:

- Construction of drainage to meet the requirements of Melbourne Water and Council, including 1 in 100-year ARI capacity road reserves and underground drainage for the 1 in 5-year ARI storm event as needed.
- A stormwater detention volume of approximately 25,200 m³ will be required to provide the appropriate detention for the proposed development. This volume will be contained within the proposed retarding basins located throughout the site, to cater for the designated subcatchment areas.
- The stormwater quality treatment system required to meet BPEMG standards will consist of a swale, 2 wetlands and 3 sedimentation basins with a total combined area of 13,160 m². One of the wetlands and sedimentation basins will be located within the western portion of 379 Lang Lang Poowong Road (adjacent to the current location of the existing dam) and another wetland is to be located on the east of this lot to service the future residential development. The swale will bisect these two assets and will convey flows from the south of this lot to the north. In addition, a 3,500 m² bioretention basin will be located on 30 Glovers Road to provide treatment to the western catchment.
- Construction of WSUD assets and detention basins to meet the overall stormwater quality treatment and detention requirements.

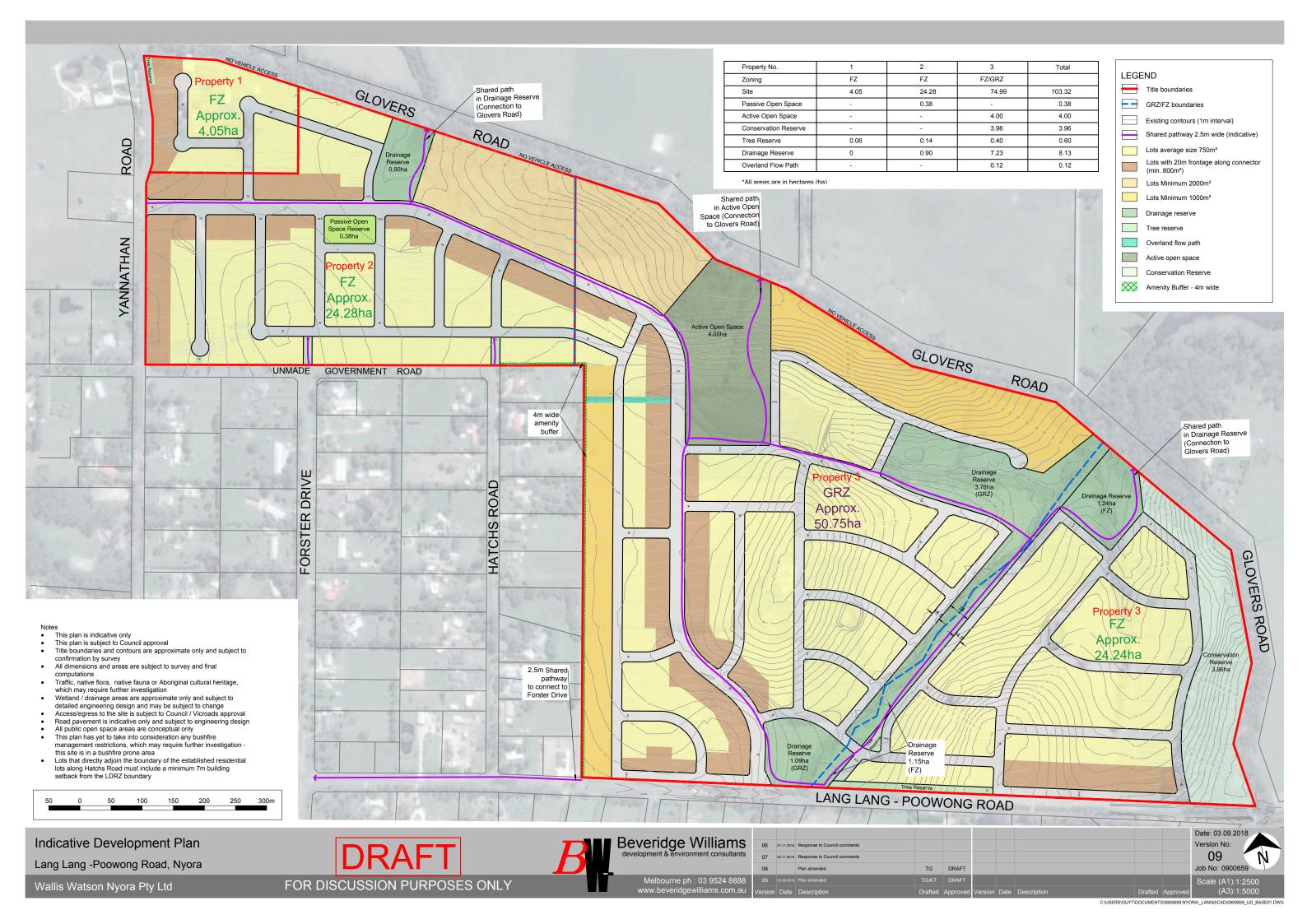
The above strategy can be implemented and all Melbourne Water and Council's development requirements can be achieved, with no net effect on the downstream properties.

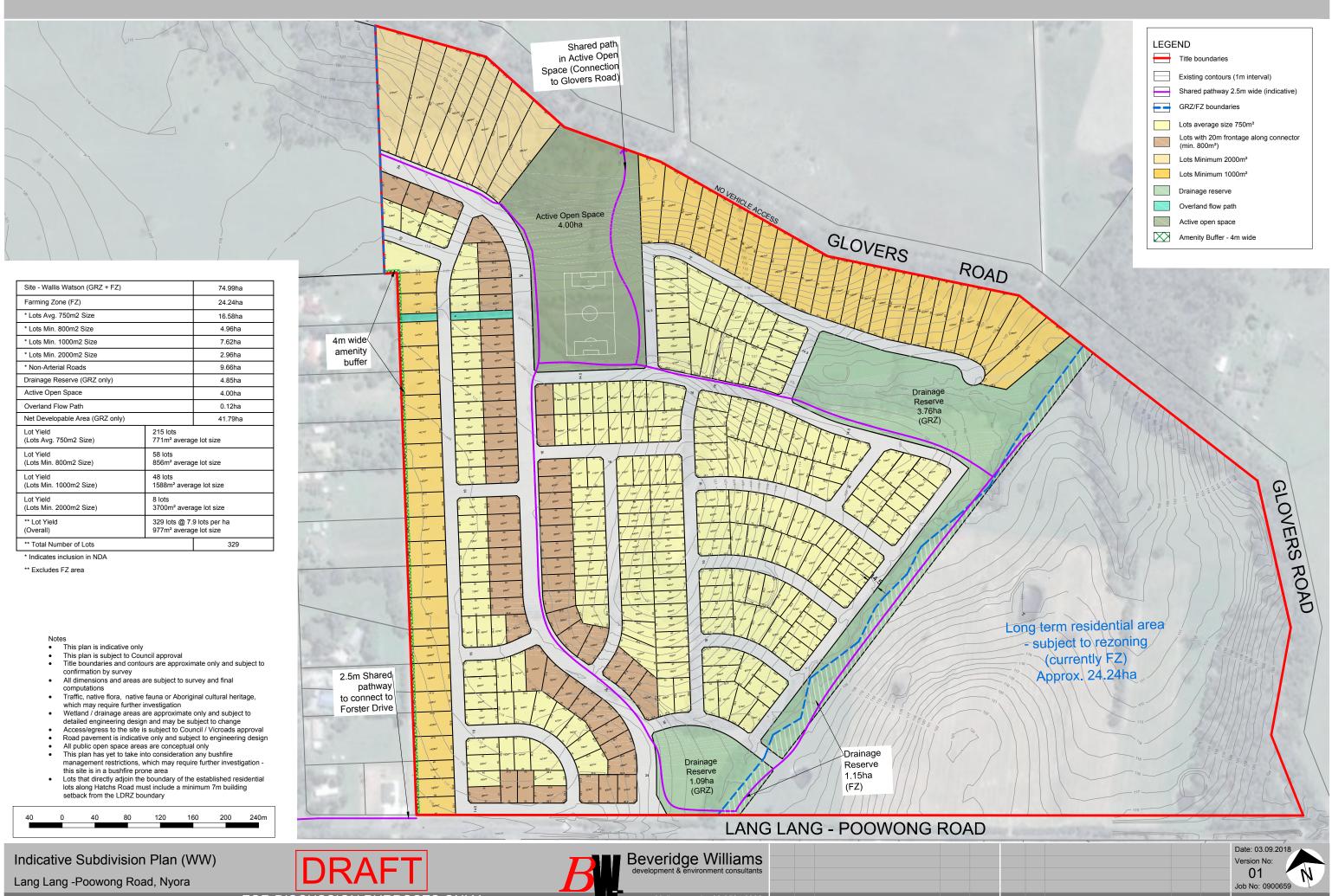
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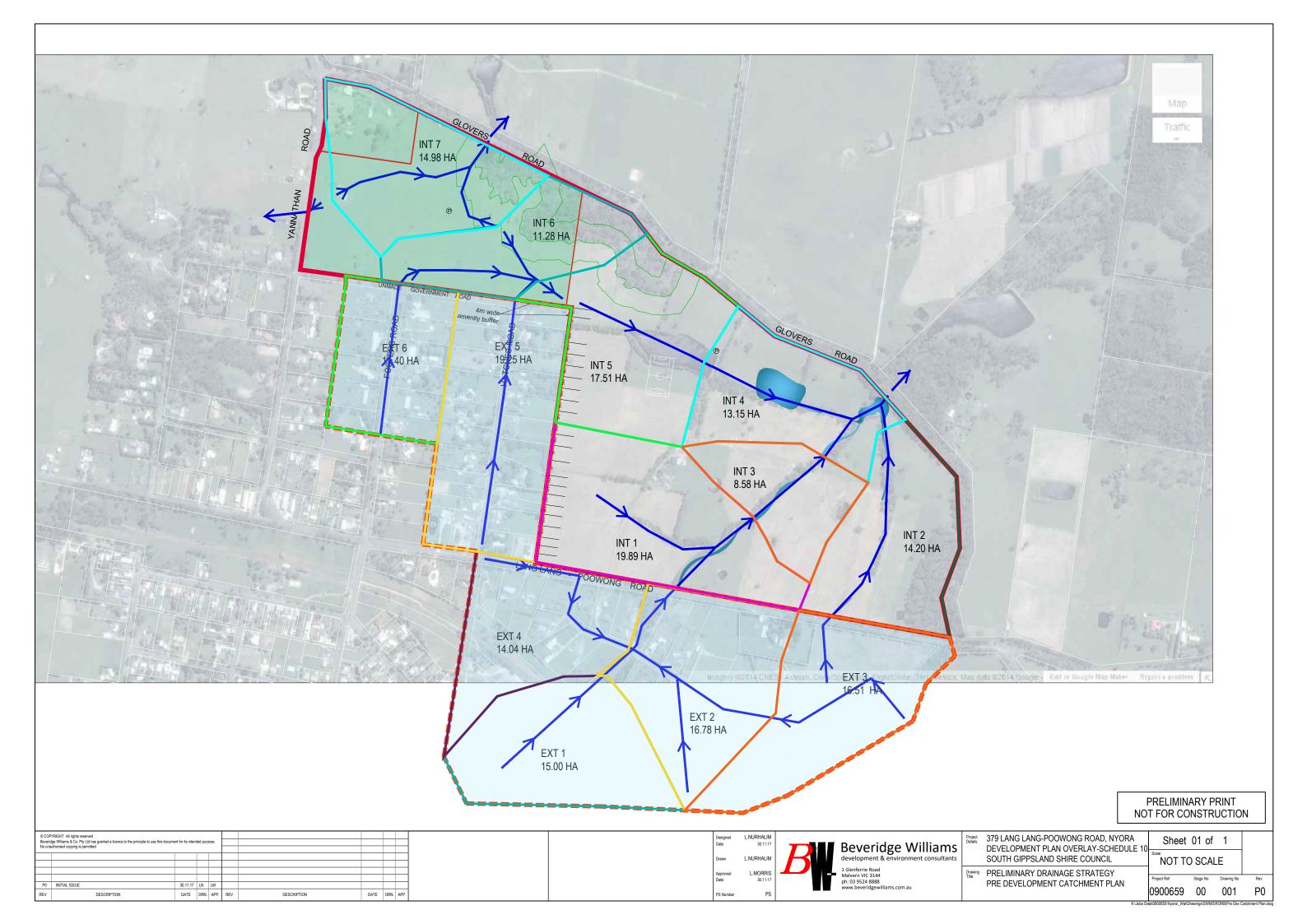


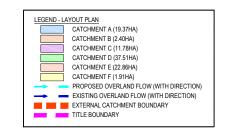
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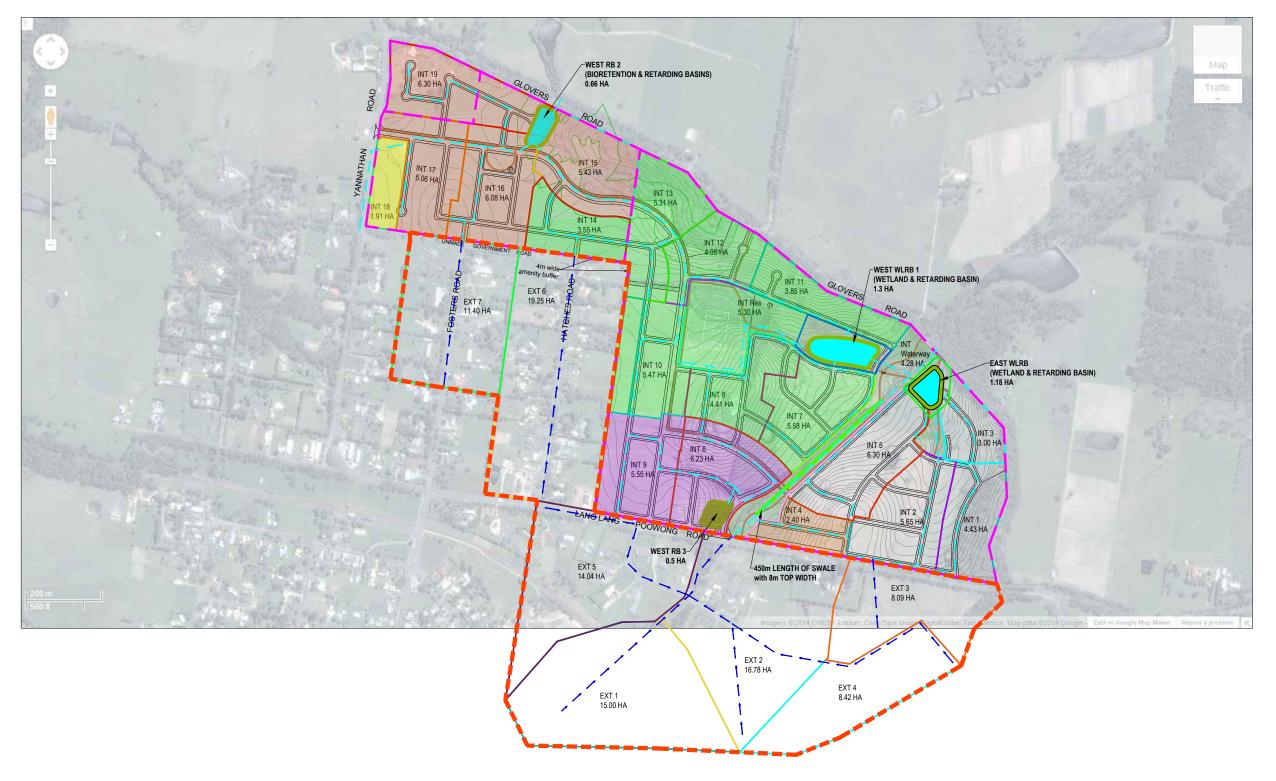
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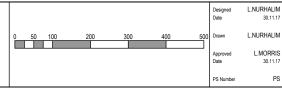






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t	379 NYORA-POOWONG ROAD, NYORA
	DEVELOPMENT PLAN OVERLAY-SCHEDULE
	SOUTH GIPPSLAND SHIRE COUNCIL

Drawing Title PRELIMINARY DRAINAGE STRATEGY

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# RORBWin Batch Run Summary

Program version 6.32 (last updated 3rd September 2017) Copyright Monash University and Hydrology and Risk Consulting

Date run: 12 Sep 2018 16:47

Catchment file : K:\Jobs Data\0900659 Nyora\\_Wat\Models\RORB\Pre Dev\Pre Dev Catch.catg

Rainfall location: Nyora

Temporal pattern : AR&R87 Volume 2 for zone 1 (filtered)

Spatial pattern : Uniform

Areal Red. Fact. : Based on ARR87 Bk II, Figs 1.6 and 1.7

Loss factors : Constant with ARI

Parameters: kc = 1.30 m = 0.80

Loss parameters Initial loss (mm) Runoff coeff. 10.00 0.60

Peak Description
01 Calculated

01 Calculated hydrograph, Ext East

02 Calculated hydrograph, East Outlet

03 Calculated hydrograph, Ext West

04 Calculated hydrograph, West Outlet

05 Calculated hydrograph, Overall Outlet

Run	Dur	ARI	Rain(mm)	ARF	Peak0001	Peak 0002	Peak 0003	Peak 0004	Peak 0005
1	6m	100y	15.70	0.99	3.8178	0.9139	1.1005	0.3462	1.1870
2	10m	100y	20.54	0.99	5.4649	1.7334	1.4419	0.6694	2.2688
3	15m	100y	24.76	0.99	6.3039	2.5160	1.6259	0.9765	3.3025
4	2 0m	100y	27.93	0.99	6.5481	3.1271	1.5234	1.2072	4.1029
5	25m	100y	30.48	0.99	6.9450	3.6307	1.6203	1.4094	4.7859
6	3 Om	100y	32.59	0.99	6.3828	4.0397	1.4962	1.5505	5.3329
7	45m	100y	37.40	1.00	5.7723	4.9881	1.3164	1.8047	6.6142
8	1h	100y	40.91	1.00	6.6956	5.6730	1.5212	1.8827	7.3559
9	1.5h	100y	48.03	1.00	6.6078	6.1668	1.4372	1.8374	7.8228
10	2h	100y	53.63	1.00	7.0040	6.4364	1.6315	1.9308	8.1793
11	3h	100y	62.47	1.00	4.2230	6.1694	0.9192	1.7660	7.7808
12	4.5h	100y	72.67	1.00	4.7298	5.9757	0.9482	1.8819	7.6222
13	6h	100y	80.93	1.00	3.4018	6.3794	0.6915	1.7951	8.0154
14	9h	100y	94.26	1.00	3.2596	6.1117	0.6098	1.6993	7.6788
15	12h	100y	105.07	1.00	3.0603	5.5183	0.6054	1.4958	7.0141
16	18h	100y	122.81	1.00	1.9773	3.9981	0.3939	1.0478	5.0459
17	24h	100y	136.87	1.00	2.0585	4.7503	0.3980	1.2488	5.9991
18	3 Oh	100y	148.48	1.00	1.5911	3.4229	0.3070	0.8488	4.2717
19	36h	100y	158.30	1.00	1.4470	3.5323	0.2765	0.8965	4.4288
20	48h	100y	174.00	1.00	1.7305	3.3967	0.3212	0.9627	4.3594
21	72h	100y	194.97	1.00	1.1520	2.2214	0.2226	0.5855	2.7405

Elapsed Run Time (hh:mm:ss) = 00:00:02

# RORBWin Batch Run Summary

Program version 6.32 (last updated 3rd September 2017) Copyright Monash University and Hydrology and Risk Consulting

Date run: 07 Sep 2018 10:34

Catchment file : K:\Jobs Data\0900659 Nyora\ Wat\Models\RORB\Post Dev\Final\Post Dev Catch Opt 2.catg

Rainfall location: Nyora

Temporal pattern : AR&R87 Volume 2 for zone 1 (filtered)

Spatial pattern : Uniform

Areal Red. Fact. : Based on ARR87 Bk II, Figs 1.6 and 1.7

Loss factors : Constant with ARI

Parameters: kc = 1.15 m = 0.80

Loss parameters Initial loss (mm) Runoff coeff. 10.00 0.60

Peak Description

Run

01 Special storage: West RB 1 - Outflow
02 Special storage: West RB 1 - Inflow
03 Special storage: East RB - Outflow
04 Special storage: East RB - Inflow
05 Special storage: West RB 3 - Outflow
06 Special storage: West RB 3 - Inflow
07 Calculated hydrograph, Ext East

07 Calculated hydrograph, Ext East 08 Calculated hydrograph, East Outlet

09 Special storage : West RB 2 - Outflow 10 Special storage : West RB 2 - Inflow

11 Calculated hydrograph, West Outlet 2

12 Calculated hydrograph, West Outlet 1

13 Calculated hydrograph, Outlet

ARI Rain(mm)

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1	6m	100y	15.70	0.99	0.4247	4.7988	0.0847	4.1653	0.0746	2.4907	4.7776	1.3584	0.5530	4.9125	0.5530	0.1725	1.7269
2	10m	100y	20.54	0.99	0.8136	6.1486	0.1563	4.0347	0.1363	2.4816	6.4286	2.3823	0.9474	5.7310	0.9474	0.1775	3.1596
3	15m	100y	24.76	0.99	1.1590	6.7138	0.2295	4.4074	0.1486	2.6838	7.4666	3.3282	1.2878	6.3172	1.2878	0.1945	4.4564
4	20m	100y	27.93	0.99	1.4171	6.8539	0.2482	4.3345	0.1878	2.6742	7.5850	4.0690	1.4852	6.0898	1.4852	0.1838	5.4426
5	25m	100y	30.48	0.99	1.6097	7.1432	0.2713	4.1464	0.2174	2.5185	8.1241	4.5954	1.5616	5.9515	1.5616	0.1680	6.0617
6	30m	100y	32.59	0.99	1.7290	6.6301	0.3098	3.8936	0.2382	2.3736	7.4808	4.9822	1.6083	5.5347	1.6083	0.1574	6.4997
7	45m	100y	37.40	1.00	1.9629	5.9541	0.3815	3.4940	0.2808	2.1484	6.7342	5.6803	1.7011	5.1186	1.7011	0.1526	7.3886
8	1h	100y	40.91	1.00	2.0676	6.7150	0.4160	3.2380	0.2976	1.9515	7.8709	6.0125	1.8724	5.3401	1.8724	0.1382	7.8293
9	1.5h	100y	48.03	1.00	2.1144	6.6162	0.4563	3.2975	0.3182	1.9426	7.7015	5.9384	1.7090	5.0366	1.7090	0.1338	7.6522
10	2h	100y	53.63	1.00	2.1755	6.9557	0.4748	3.5593	0.3283	2.1569	8.3031	6.1775	1.8267	5.6154	1.8267	0.1458	8.0164
11	3h	100y	62.47	1.00	2.1208	4.4663	0.4948	2.3246	0.3344	1.4055	4.8546	5.8242	1.6309	3.5350	1.6309	0.0987	7.5105
12	4.5h	100y	72.67	1.00	2.1199	4.9075	0.4957	1.9041	0.3330	1.1530	5.5051	6.0983	1.7874	3.2763	1.7874	0.0820	7.8404
13	6h	100y	80.93	1.00	2.2033	3.6156	0.5012	1.3782	0.3428	0.8226	3.9401	5.9191	1.6169	2.2751	1.6169	0.0549	7.6191
14	9h	100y	94.26	1.00	2.2291	3.3822	0.6332	1.2024	0.3808	0.7227	3.8499	6.0021	1.5582	2.0878	1.5582	0.0508	7.6324
15	12h	100y	105.07	1.00	2.0430	3.1531	0.5165	1.2529	0.3419	0.7496	3.6306	5.5095	1.4674	2.0657	1.4674	0.0510	7.0561
16	18h	100y	122.81	1.00	1.5712	2.0298	0.4559	0.8280	0.2857	0.4959	2.3537	4.0763	0.9992	1.3498	0.9992	0.0336	5.1339
17	24h	100y	136.87	1.00	1.8250	2.1129	0.4993	0.8200	0.3250	0.4906	2.4707	4.8503	1.2112	1.3577	1.2112	0.0334	6.0919
18	30h	100y	148.48	1.00	1.2893	1.6406	0.4351	0.6245	0.2663	0.3748	1.9551	3.5134	0.8534	1.0551	0.8534	0.0261	4.3302
19	36h	100y	158.30	1.00	1.3814	1.4872	0.4320	0.5679	0.2703	0.3397	1.7518	3.6642	0.8417	0.9456	0.8417	0.0232	4.5465
20	48h	100y	174.00	1.00	1.4098	1.7856	0.4156	0.6394	0.2530	0.3827	2.0929	3.7573	0.9488	1.0987	0.9488	0.0266	4.6889
21	72h	100y	194.97	1.00	0.9589	1.1719	0.2657	0.4512	0.1597	0.2701	1.3884	2.3834	0.6628	0.7537	0.6628	0.0184	3.0060

ARF Peak0001 Peak0002 Peak0003 Peak0004 Peak0005 Peak0006 Peak0007 Peak0008 Peak0009 Peak0010 Peak0011 Peak0012 Peak0013

Elapsed Run Time (hh:mm:ss) = 00:00:08





# PRELIMINARY PRINT NOT FOR CONSTRUCTION

LEGEND - LAYOUT PLAN
CATCHMENT A (19.37HA)
CATCHMENT B (2.40HA)
CATCHMENT C (11.78HA)

CATCHMENT D (37.51HA) CATCHMENT E (22.86HA)

EXISTING OVERLAND FLOW (WITH DIRECTION)

EXTERNAL CATCHMENT BOUNDARY

TITLE BOUNDARY

CATCHMENT F (1.91HA)
PROPOSED OVERLAND FLOW (WITH DIRECTION)

